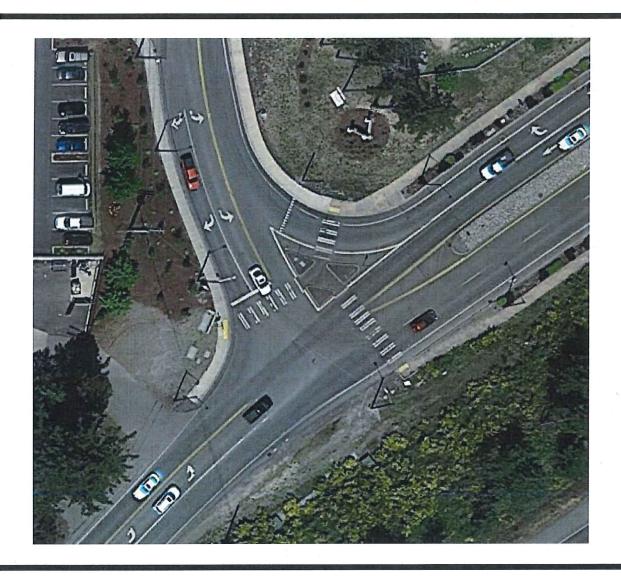
NW Greaves Way at Old Frontier Road NW Traffic Study





Traffic Division Kitsap County Public Works

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CERTIFICATION

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned, whose seal, as a professional engineer licensed to practice as such, is affixed below.



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Approved by Jeff Shea, P.E., Traffic Engineer

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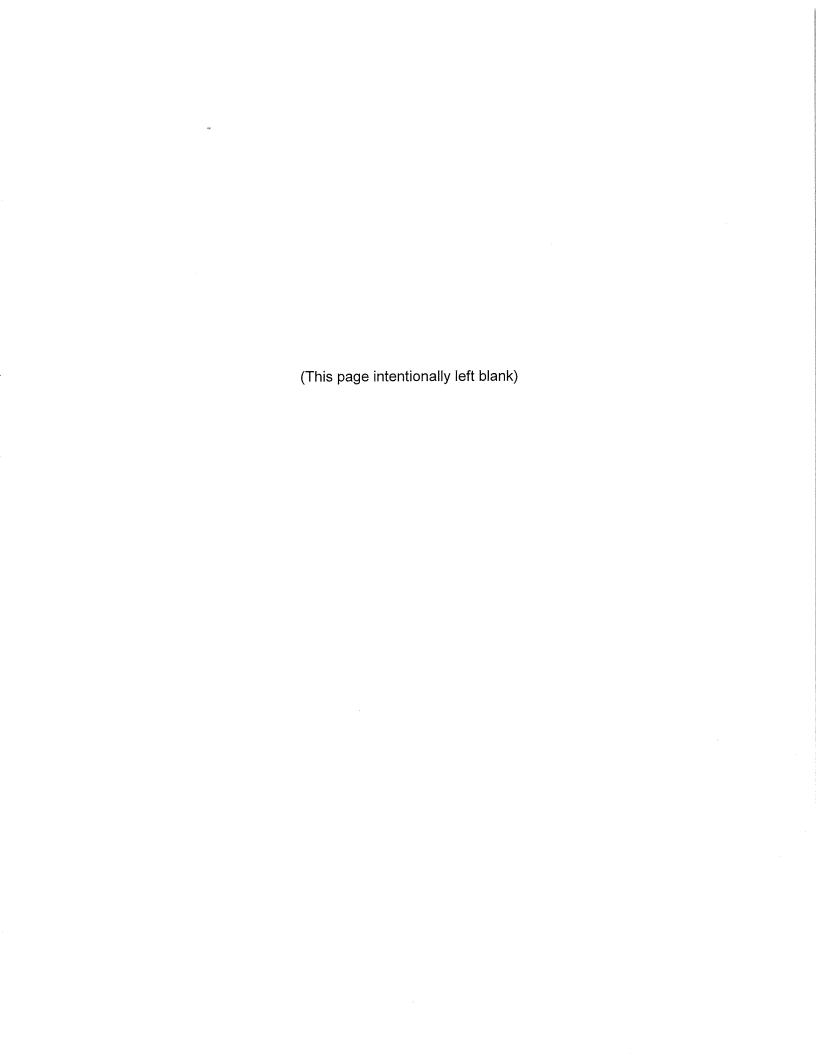


Table of Contents

E	(ECL	JTIVE S	UMMARY	1
1	INT	RODUC	TION	2
2	PRO	DJECT	DESCRIPTION	2
	2.1 2.2		SEE/PEDESTRIAN/TRANSIT	
3	RO	ADWAY	CHARACTERISTICS	4
	3.1 3.2		AY DESCRIPTIONSECTION DESCRIPTION	
4	TRA	AFFIC C	OUNT DATA	6
	4.1 4.2 4.3	DESIGN	NT AND FUTURE ADT VOLUMES	7
5	TRA	AFFIC S	AFETY	9
	5.1	Collis	ON TYPES AND FREQUENCIES	9
6	TRA	AFFIC A	NALYSIS	9
	6. 6.	INTERSI 2.1 No-E 2.2 Signs	ECTION CONTROL WARRANTS ECTION LOS Build LOS	9 10 11
7	DES	SIGN EL	EMENTS	12
	7.1	POTENT	FIAL DESIGN ISSUES	12
8	COI	NCLUSI	ON	12
9	REF	EREN	CES	14
ΑF	PEN	IDIX A.	VEHICLE VOLUME/CLASSIFICATION/SPEED DATA	A
ΑF	PEN	IDIX B.	PEAK HOUR TURNING MOVEMENT COUNT DATA	B
			COLLISION DIAGRAM	
ΑF	PEN	IDIX D.	SIGNAL WARRANT WORKSHEET	D
			AWSC ANALYSIS WORKSHEETS	
			SIGNALIZED ANALYSIS WORKSHEETS	
ΑF	PEN	IDIX G.	ROUNDABOUT ANALYSIS WORKSHEETS	G
ΑF	PEN	IDIX H.	ROUNDABOUT CONCEPTUAL DESIGN	H

List of Figures

<u> </u>	
Figure 1 – Vicinity Map	2
Figure 2 - Zoning Map	3
Figure 3 – Regional Bicycle Route Map	4
Figure 4– Roadway Orientation	5
Figure 5 – Greaves Way at Old Frontier Road Intersection	6
Figure 6 – Existing PM Peak Hour Volumes	8
Figure 7 – 2043 PM Peak Hour Volumes	8
List of Tables	7
Table 1 – Existing and Future ADT Volumes	7
Table 2 – HCM Intersection LOS Standards	10
Table 3 – SIDRA LOS Standards	10
Table 4 – No-Build AWSC Intersection LOS Summary	11
Table 5 – Signalized Intersection LOS Summary	11
Table 6 – Roundabout LOS Summary	12
List of Abbreviations	

AASHTO	American Association of State Highway Transportation Officials
CMF	Crash Modification Factor
FHWA	Federal Highway Administration
HSM	Highway Safety Manual
LOS	Level of Service
MUTCD	Manual on Uniform Traffic Control Devices
NCHRP	National Cooperative Highway Research Program
WSDOT	Washington State Department of Transportation

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Executive Summary

NW Greaves Way was constructed in 2010. At the time of the NW Greaves Way construction the intersection did not warrant signalization and was designed for future signalization with interim stop control on the north leg. The intersection of NW Greaves Way at Old Frontier Road NW was converted to all-way stop control in 2018 based on an engineering study. The MUTCD signal warrant analysis performed as part of the engineering study for the intersection control conversion showed the intersection met Warrant 1 – 8 Hour, Warrant 2 – 4 Hour, Warrant 3 – Peak Hour, and Warrant 7 – Crash Experience. The intersection is listed as number 1 on the Kitsap County's 2014-2018 Intersection Safety List (up from #4 on the 2013-2017 list).

The 2019-2024 Six Year Transportation Improvement Program included the Greaves Way NW / Old Frontier Road NW Project as priority number 40. The project has a budget of \$800,000 with a construction date of April 2023. The project is described as a traffic signal installation. The County has a policy to evaluate other traffic control devices including a roundabout prior to a traffic signal installation. This traffic study evaluates the intersection operations under both signal and roundabout design.

The intersection was evaluated under existing and design year 2043 traffic conditions. The evaluation included analysis of signalized and single lane roundabout control options. The analysis concluded that the operational performance of the NW Greaves Way at Old Frontier Road intersection would be improved by signalization or roundabout conversion. Both traffic control measures would reduce per-vehicle delay and reduce frequency and severity of angle-entry and left versus opposing through collisions. Whether the preferred alternative is a signal or a roundabout, this report recommends:

- Exterior lanes should be 12 feet wide,
- Sidewalks should be 6 feet wide and installed on all sides of the intersection,
- Bicycle lanes should be 5 feet wide and installed on all legs,
- Intersection lighting should follow ANSI/IES recommendations,
- If the intersection is converted to a roundabout, then the report recommends a single lane compact design.

The decision to convert the intersections from "T" intersections with stop control to roundabouts should consider:

- Safety- reductions in fatal and injury collisions
 - Pedestrians roundabouts offer shorter crossing distances and pedestrians only cross one direction of traffic at a time
- Maintenance costs no signal hardware or equipment maintenance with roundabouts
- Traffic calming reduced vehicular speeds with roundabouts

1 Introduction

NW Greaves Way was constructed in 2010 and the intersection of NW Greaves Way and Old Frontier Road NW as designed for future signalization with interim stop control on the north leg. The intersection of NW Greaves Way at Old Frontier Road NW was converted to all-way stop control in 2018 based on an engineering study. The MUTCD signal warrant analysis performed as part of the engineering study for the intersection control conversion showed the intersection met Warrant 1 – 8 Hour, Warrant 2 – 4 Hour, Warrant 3 – Peak Hour, and Warrant 7 – Crash Experience. The intersection is listed as number 1 on the Kitsap County's 2014-2018 Intersection Safety List (up from #4 on the 2013-2017 list).

2 Project Description

The 2019-2024 Six Year Transportation Improvement Program (TIP) included the Greaves Way NW / Old Frontier Road NW Project as priority number 40. The project is described as a traffic signal installation. The County has a policy to evaluate other traffic control devices including a roundabout prior to a traffic signal installation. This traffic study evaluates the intersection operations under both signal and roundabout design.

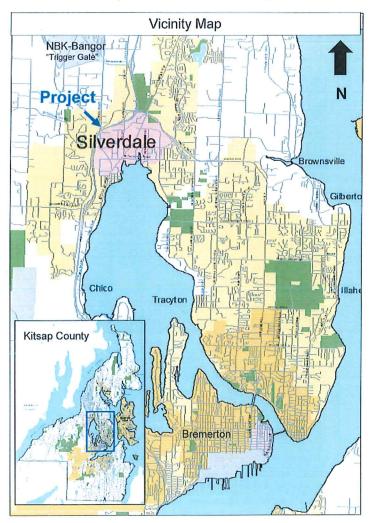


Figure 1 - Vicinity Map

2.1 Land Use

The project area is in the Silverdale Urban Growth Area (UGA) in Section 17, Township 25 North, and Range 1 East of the Willamette Meridian. The intersection is within the Waaga Way Town Center District in the adopted Silverdale Design Standards. The area surrounding the intersection is zoned commercial and industrial and has seen significant development within the last few years. Figure 2 is a zoning map of the location and the surrounding area.

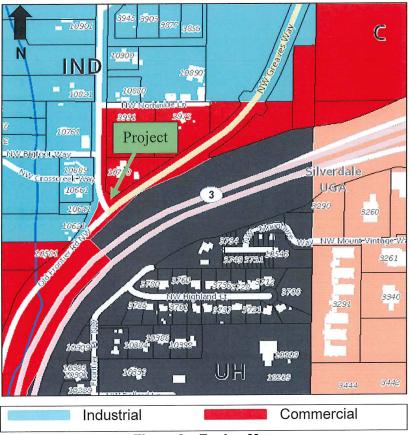


Figure 2 - Zoning Map

2.2 Bicycle/Pedestrian/Transit

The Kitsap County Non-Motorized Facilities Plan identifies opportunities for bikers, walkers, and all persons with "connections within communities" and "connections between communities". The Plan goals are:

- •Recognize mobility needs of everyone
- •Identify differences between rural and urban areas
- •Make connections within communities, i.e. schools, parks, and services
- •Make connections between communities within Kitsap County
- Promote recreational uses

The Kitsap County Non-Motorized Facility Plan identifies NW Greaves Way and Old Frontier Road NW as non-motorized routes. These routes combine to be the primary north-south connecting route west of SR 3 for the County. A map of the routes, extracted from the plan, is shown as Figure 3.

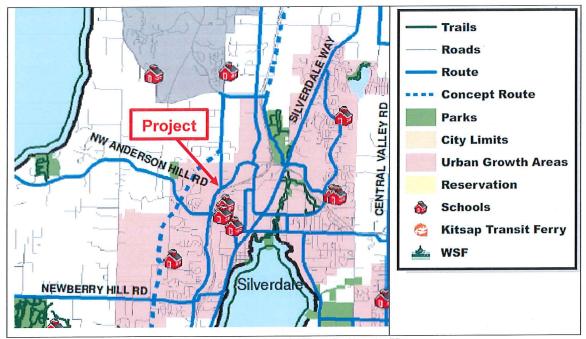


Figure 3 - Regional Bicycle Route Map

Kitsap Transit's routed buses do not serve the study intersection. Approximately ½ mile east of the intersection is a commercial shopping center and the Kitsap Transit's Silverdale Transit Center. A 2017 traffic count showed no pedestrians crossing at the intersection during the PM peak hour and a 2016 traffic count showed 3 pedestrian crossings during the PM peak hour. No studies were conducted to count average daily pedestrian volumes.

3 Roadway Characteristics

3.1 Roadway Descriptions

NW Greaves Way was constructed in 2010. Prior to the NW Greaves Way construction, Old Frontier Road curved to the north at the location of the current intersection. Greaves Way provides a direct connection to State Route (SR) 3 from the west side of Silverdale. The construction of NW Greaves Way allowed commercial development of the adjacent land.

NW Greaves Way (Road Log ID 57768) is a paved urban minor arterial which generally runs in an east-west orientation. NW Greaves Way forms the study intersection's east leg. It begins at Old Frontier Road NW and terminates State Route 303. It consists of two 11-foot travel lanes in each direction separated by a planted median. There are 5-foot bike lanes on both sides of NW Greaves Way and 6-foot sidewalks separated from the travel lanes by planted buffer strips. The posted speed on NW Greaves Way is 35 mph.

Old Frontier Road NW (Road Log ID 57810) is a paved urban major collector. It forms the intersection's west and north legs. Old Frontier Road NW begins at NW Anderson Hill Road and runs in a southwest to northeast direction then makes sharp turn to the north at the intersection with NW Greaves Way and terminates at NW Half Mile Road. It consists of one 12-foot travel lane in each direction. West of the study intersection Old Frontier Road NW has 3-foot paved shoulders on each side of the roadway delineated by white edge lines and a posted speed of 35 mph. North of the study intersection there are 6-foot sidewalks adjacent to the travel lanes on both sides of the road and it has a posted speed of 40 mph. One full movement commercial driveway exists within the intersection functional area. It is located on Old Frontier Road NW approximately 60 feet west of the intersection.

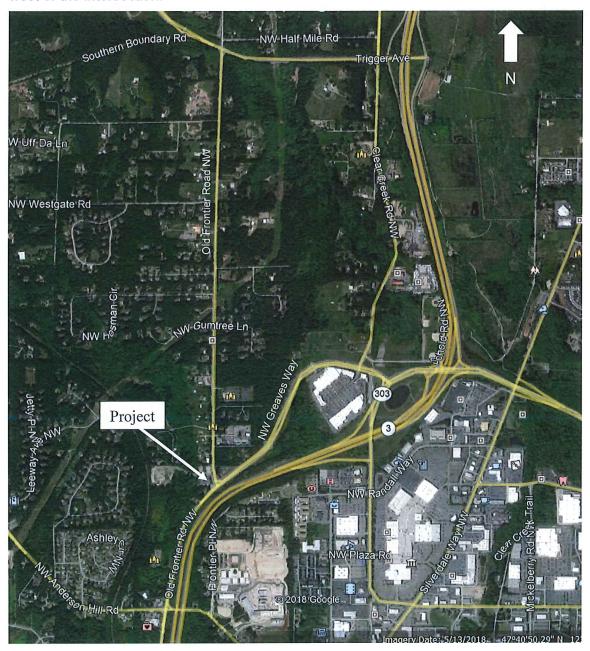


Figure 4 - Roadway Orientation

3.2 Intersection Description

The junction of NW Greaves Way and Old Frontier Road form a three-leg intersection with stop control on all approaches. The intersection corresponds to milepost 0.00 on NW Greaves Way and milepost 0.424 on Old Frontier Road NW. The north leg consists of a separate left turn lane and separate right turn lane. The west leg consists of a separate left turn lane and a separate through lane. The east leg consists of a separate through lane and a right-turn slip lane controlled by a yield sign. Pedestrian facilities at the study intersection include marked crosswalks on the east and north legs. Within the intersection limits, sidewalk is present on both sides of Greaves Way and on both sides of Old Frontier Road north of the intersection. There is no sidewalk, only 3-foot paved shoulders on Old Frontier Road west of the intersection. Bike lanes exist only on NW Greaves Way. Figure 5 is an aerial view of the study intersection.



Figure 5 - Greaves Way at Old Frontier Intersection

4 Traffic Count Data

The daily traffic count data were determined by road tube counts. Traffic count data for the Old Frontier Road NW legs were collected on March 29, 2017. Traffic count data for the NW Greaves Way leg was collected on March 29, 2018. Counts were conducted on a mid-week day, which better represents an average daily traffic condition. Copies of the

Daily Vehicle Volume and Daily Vehicle Classification are found in Appendix A of this report.

4.1 Current and Future ADT Volumes

Average daily traffic (ADT) volumes for the study intersection are shown in Table 1 for both current and design year conditions. The future ADT volumes were calculated by applying a historical 2% annual growth rate to the 2017 volumes.

Table 1 - Existing and future ADT volumes

Leg	Existing Year ADT	Design Year 2043 ADT
North	4,652	7,769
West	8,748	14,609
East	8,590	14,345

4.2 Design Vehicle

AASHTO's A Policy on Geometric Design of Highways and Streets defines standard characteristics of 19 design vehicles within the general vehicle classes. From the AASHTO list, the following vehicles have been selected as the minimum design vehicle for each functional classification of Kitsap County roadways:

Principal ArterialWB-40 (Intermediate Semi-trailer)

Minor ArterialWB-40 (Intermediate Semi-trailer)

CollectorSU (Single Unit Truck)

Local AccessSU (Single Unit Truck)

Based on the roadway functional classification a WB-40 (Intermediate Semi-trailer) is an appropriate design vehicle for the intersection. Old Frontier Road NW is classified as an urban major collector. NW Greaves Way is classified as an urban minor arterial. However, a design of a roadway should consider the dimensions and operating characteristics of the vehicles traveling on the roadway. Kitsap County Road Standards state that the road's geometry must accommodate the physical dimensions and turning radius for the largest design vehicle likely to use the road with considerable frequency. Of the 992 total daily trucks entering the intersection, 980 (98.8%) were single-unit trucks of which 925 (93.2%) were two axel single unit trucks. In addition, 9 (0.9%) of the total trucks were double-unit trucks, and 3 (0.3%) of the total trucks were multi-unit trucks.

4.3 Current and Future Turning Movement Counts

Turning movement counts were collected on October 26, 2017. These counts were used to represent existing conditions. Figure 6 shows the existing PM peak turning movement counts for the study intersection. The PM peak hour is between 4:30 PM and 5:30 PM. Appendix B includes a copy of the turning movement counts report.

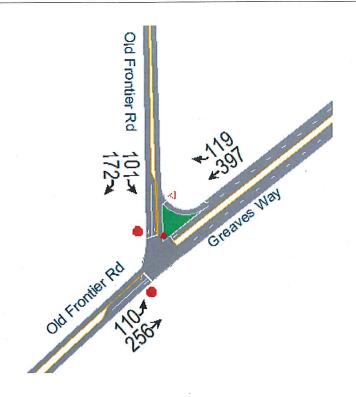


Figure 6 – Existing PM Peak Hour Volumes

Base on a 20-year design life and a construction date of 2023, the design year 2043 background turning movement counts were derived by applying a historical 2.0% annual growth rate to existing volumes. Figure 7 shows the 2043 PM peak hour traffic volumes used in this report's analysis.

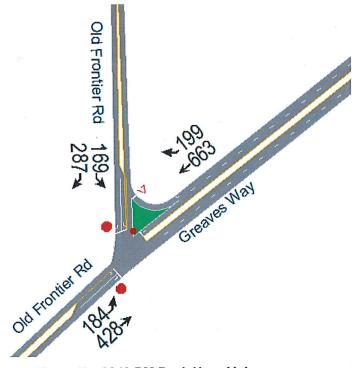


Figure 7 – 2043 PM Peak Hour Volumes

5 Traffic Safety

The current 2014-2018 Traffic Safety Intersection List ranks the intersection of NW Greaves Way and Old Frontier Road NW as 1 out of 81. In the previous 2013-2017 Traffic Safety Intersection List, the study intersection was ranked 4 of 78. All reported motor vehicle collisions within the project limits from January 1, 2014 through December 31, 2018 were analyzed. Collision data was retrieved using Mobility, a database utility maintained by WSDOT's County Road Administration Board. The collisions were analyzed for their type and frequency. A collision diagram is included with this report as Appendix C.

5.1 Collision Types and Frequencies

A total of 29 collisions occurred at the project intersection 2014-2018. Of the 29 collisions:

- 1 was fatal,
- 12 were injury collisions,
- 16 were property damage only (PDO) collisions.

The most frequently cited contributing circumstance to crashes was "failure to grant right of way" (total of 17), followed by inattention (total of 5), and speed (total of 2). Of the 29 collisions analyzed:

- 18 were left-turn vs. opposing through,
- 5 were angle-entry,
- 4 were fixed object,
- 1 were rear-end;
- 1 was vehicle hits pedestrian.

6 Traffic Analysis

6.1 Intersection Control Warrants

An existing intersection signal control warrant analysis was performed. The 2009 Manual on Uniform Traffic Control Devices (MUTCD) prescribes nine warrants for traffic signal control installation. The intersection currently meets Warrant 1 – 8 Hour, Warrant 2 – 4 Hour, Warrant 3 – Peak Hour, and Warrant 7 – Crash Experience. Appendix D includes the traffic signal warrant analysis worksheet.

6.2 Intersection LOS

Signalized intersection Level of Service (LOS) were evaluated using Synchro Studio 10 by Trafficware. The method used for evaluating intersection LOS is described in the Highway Capacity Manual (HCM). Table 2 shows the control delay and corresponding HCM LOS for signalized intersections.

LOS can be characterized for the entire intersection, each intersection approach, and each lane group. Control delay alone is used to characterize LOS for the entire

intersection or an approach. Control delay and volume to capacity ratios are used to characterize LOS for a lane group.

Table 2 – HCM Intersection LOS Standards

Signalized Delay	LOS by Volume-to-Capacity Ratio						
(s/veh)	v/c ≤ 1.0	v/c ≥ 1.0					
≤ 10	Α	F					
> 10 - 20	В	F					
> 20 - 35	С	F					
> 35 - 55	D	F					
> 55 - 80	Е	F					
> 80	F	F					

Roundabout operations for the intersection were analyzed using SIDRA Intersection 8.0 by Akcelik and Associates Pty Ltd. The analysis method followed WSDOT guidelines. Table 3 shows the SIDRA method level of service definitions base on delay and degree of saturation.

Table 3 - SIDRA LOS Standards

LOS	Control Delay per vehicle in seconds (d)*	Degree of saturation (v/c ratio) (x)				
Α	d ≤10	$0 < x \le 0.85$				
В	10 < d ≤ 20	$0 < x \le 0.85$				
С	20 < d ≤ 35	$0 < x \le 0.85$				
. D	30 < d ≤ 50	$0 < x \le 0.85$				
D	0 < d ≤ 50	$0.85 < x \le 0.95$				
E	50 < d ≤ 70	$0 < x \le 0.95$				
Е	0 < d ≤ 70	0.95 < x ≤ 1.00				
F	70 < d	1.00 < x				

^{*}Only the average delay value is considered in determining LOS for approaches and the intersection

6.2.1 No-Build LOS

Intersection delay and LOS is not defined by the HCM for unsignalized all-way stop controlled (AWSC) intersections. The HCM methodology calculates delay and LOS for individual movements. Analysis of existing conditions indicates that intersection individual movements are operating acceptably. A detailed LOS analysis for the study intersection under the AWSC is included with this report as Appendix E. Table 4 summarizes the existing and future LOS for the intersection's critical movements.

Table 4 – No-build AWSC Intersection LOS Summary

据对表现代表示 化水流 医皮肤 有强力	Existing			2043			
			Delay			Delay	
Movement	LOS	V/C	(s/veh)	LOS	V/C	(s/veh)	
Greaves Way (East Leg)							
Thru	D	.78	27.5	F	1.59	300	
Right	В	.28	10.5	С	0.60	19.7	
Old Frontier Rd (North Leg)							
Left	В	.26	13.1	С	.52	20.4	
Right	В	.37	12.9	D	.76	27.8	
Old Frontier Rd (West Leg)							
Left	В	.27	12.3	С	.56	20.6	
Thru	С	.53	16.2	F	1.11	81.2	

6.2.2 Signalized LOS

The project intersection was analyzed under signal control for existing and 2043 PM peak hour traffic volumes. All the lane groups and approaches operated at acceptable LOS D or better under future traffic volumes and existing geometry. Table 5 gives a summary of intersection operations. Detailed intersection LOS analysis worksheets are included in Appendix F.

Table 5 - Signalized Intersection LOS Summary

		Existi	ng	2043			
Lane Group	LOS	Delay (s/veh)	95 th Percentile Queue (ft)	LOS	Delay (s/veh)	95 th Percentile Queue (ft)	
Greaves Way (East) Through	В	13.1	161	С	34.4	399	
Old Frontier Rd (North) Left Right	B B	13.5 16.3	71 73	C D	25.5 51.6	135 205	
Old Frontier Rd (West) Left Through	B A	18.5 4.3	85 99	D A	49.9 6.7	135 248	
Intersection	В	12.2		С	31.5		

6.2.3 Roundabout LOS

Per Kitsap County Policy (PW-33.31.01), roundabouts should be considered as an alternative when an intersection meets all-way stop and/or signal warrants under existing traffic conditions or under projected traffic volumes one year after project construction, or if an accident pattern suggests that a roundabout would be an appropriate countermeasure.

Roundabouts typically operate better when the traffic volume demands for the approach legs are about equal. Roundabout operations for the intersection of NW Greaves Way and Old Frontier Road NW were analyzed using SIDRA Intersection 8 by Akcelik and Associates Pty Ltd. The analysis method followed WSDOT guidelines. A single-lane roundabout analysis for 2043 showed the intersection operating at an acceptable LOS A.

Table 6 is a summary of the roundabout approaches and intersection LOS under existing and future conditions. The analysis details are included with this report as Appendix G.

Table 6 - Roundabout LOS Summary

		Existi	ng	2043			
Approach	LOS	Delay (s/veh)	95 th Percentile Queue (ft)	LOS	Delay (s/veh)	95 th Percentile Queue (ft)	
Greaves Way (East)	Α	4.1	78	Α	5.5	207	
Old Frontier Rd (North)	Α	8.5	44	В	13.1	147	
Old Frontier Rd (West)	Α	5.4	50	Α	5.9	109	
Intersection	Α	5.5		Α	7.4		

7 Design Elements

7.1 Potential Design Issues

Turning movements projected for 2043 show that the roundabout would operate at LOS A if a single lane circulating road is in use. The Kitsap County Comprehensive Plan shows no plans to expand Old Frontier Road NW. One potential challenge to converting the study intersection to a roundabout is acquiring the needed right-of-way. Appendix H includes a figure depicting a single lane roundabout design. It appears that additional right-of-way on the south side of the intersection may be required. This right-of-way belongs to WSDOT.

A second potential design issue involves a commercial driveway access on Old Frontier Road NW west of the intersection. The driveway is within the functional area of the intersection and provides access to commercial building on a small parcel adjacent to the intersection (see Appendix H). It appears from an aerial view that the small parcel also has access to an easement across the back side of an adjoining property to a private road north of the intersection. Both parcels are owned by Old Frontier LLC.

8 Conclusion

The operational performance of the NW Greaves Way at Old Frontier Road intersection would be improved by signalization or roundabout conversion. Both traffic control measures would reduce per-vehicle delay and reduce frequency and severity of angle-entry and left versus opposing through collisions. Whether the preferred alternative is a signal or a roundabout, this report recommends:

- Exterior lanes should be 12 feet wide.
- Sidewalks should be 6 feet wide and installed on all sides of the intersection,
- Bicycle lanes should be 5 feet wide and installed on all legs,
- Intersection lighting should follow ANSI/IES recommendations,
- If the intersection is converted to a roundabout, then the report recommends a single lane compact design.

The decision to convert the intersections from "T" intersections with stop control to roundabouts should consider:

- Safety- reductions in fatal and injury collisions
 - Pedestrians roundabouts offer shorter crossing distances and pedestrians only cross one direction of traffic at a time
- Maintenance costs no signal hardware or equipment maintenance with roundabouts
- Traffic calming reduced vehicular speeds with roundabouts

9 References

- 1. Design Manual. Washington State Department of Transportation, February 2019.
- 2. "FHWA Vehicle Classification." Federal Highway Administration, November 2014 (accessed June 13, 2016). http://www.fhwa.dot.gov/policyinformation/tmguide/tmg 2013/vehicle-types.cfm.
- 3. *Highway Capacity Manual*. Transportation Research Board of the National Academies, 2010 edition.
- 4. *Kitsap County Non-Motorized Facility Plan*. Kitsap County Public Works, Port Orchard, Washington, April 2015.
- 5. *Kitsap County Road Standards*. Kitsap County Public Works, 2007 edition. http://www.kitsapgov.com/pw/pdf/Final%20Road%20Standards.pdf
- 6. *Manual on Uniform Traffic Control Devices*. Federal Highway Administration, 2009 edition.
- 7. A Policy on Geometric Design of Highways and Streets. American Association of State Highway Transportation Officials, 2010.
- 8. Roadside Design Guide. American Association of State Highway Transportation Officials, 2011.

Appendix A. Vehicle Volume/Classification/Speed Data

FHWA Vehicle Classification System

Class I Motorcycles	6	Class 7 Four or more axle, single unit	••••			
Class 2 Passenger cars		axio, single and				
			6			
a a		Class 8 Four or less axle,				
,		single trailer				
Class 3 Four tire,						
single unit		Class 9 5-Axle tractor	00 00 0			
,		semitrailer				
Class 4 Buses		Class 10 Six or more axle,				
		single trailer				
		Class I I Five or less axle, multi trailer				
Class 5 Two axle, six		Class 12 Six axle, multi-				
tire, single unit		trailer				
÷		Class 13 Seven or more axle, multi-trailer	0 00 00 00			
Class 6 Three axle, single unit			00 000 000			
			00 00 000			

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Daily Vehicle Volume Report

Study Date: Wednesday, 03/29/2017 / Thursday, 03/30/2017

Unit ID: 5

Location: Old Frontier Rd. N. of Greaves Way 110.7

	Southbound	Northbound	Total
	Volume	Volume	Volume
11:00 - 11:59	175	137	312
12:00 - 12:59	150	172	322
13:00 - 13:59	147	128	275
14:00 - 14:59	180	137	317
15:00 - 15:59	285	179	464
16:00 - 16:59	257	177	434
17:00 - 17:59	186	183	369
18:00 - 18:59	147	144	291
19:00 - 19:59	98	93	191
20:00 - 20:59	47	65	112
21:00 - 21:59	17	30	47
22:00 - 22:59	10	18	28
23:00 - 23:59	6	10	16
00:00 - 00:59	4	3	7
01:00 - 01:59	4	2	6
02:00 - 02:59	0	4	4
03:00 - 03:59	4	4	8
04:00 - 04:59	13	16	29
05:00 - 05:59	23	122	145
06:00 - 06:59	97	202	299
07:00 - 07:59	97	169	266
08:00 - 08:59	121	123	244
09:00 - 09:59	148	101	249
10:00 - 10:59	111	106	217
Totals	2327	2325	4652
AM Peak Time	11:00 - 11:59	05:49 - 06:48	06:44 - 07:43
AM Peak Volume	175	204	314
PM Peak Time	15:34 - 16:33	16:54 - 17:53	15:30 - 16:29
PM Peak Volume	305	190	492

Daily Southbound Classes Report

Study Date: Wednesday, 03/29/2017 / Thursday, 03/30/2017

Unit ID: 5

Location: Old Frontier Rd. N. of Greaves Way 110.7

г		1		".	,, <u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>	"0		40	40	#10	#11	#12	#13	Total
	#1	#2	#3	#4	#5	#6	#7	#8	#9					
11:00 - 11:59	0	141	25	0	9	0	0	0	0	0	0	0	0	175
12:00 - 12:59	0	110	29	0	10	1	0	0	0	0	0	0	0	150
13:00 - 13:59	. 0	113	24	1	9	0	0	0	0	0	0	0	0	147
14:00 - 14:59	1	132	35	. 3	9	0	0	0	0	0	0	0	0	180
15:00 - 15:59	3	199	58	1	24	0	0	0	0	0	0	0	0	285
16:00 - 16:59	1	184	38	5	28	1	0	0	0	0	0	0	0	257
17:00 - 17:59	0	145	18	0	22	0	0	1	0	0	0	0	0	186
18:00 - 18:59	1	122	19	0	5	0	0	0	0	0	0	0	0	147
19:00 - 19:59	0	83	. 8	0	7	0	0	0	0	0	0	0	0	98
20:00 - 20:59	0	42	4	0	1	0	0	. 0	0	0	0	0	0	47
21:00 - 21:59	0	16	0	0	1	0	0	0	0	0	0	0	0	17
22:00 - 22:59	0	7	2	0	1	0	0	0	0	0	0	0	0	10
23:00 - 23:59	0	6	0	0	0	0	0	0	0	0	0	0	0	6
00:00 - 00:59	0	2	0	0	2	0	0	0	0	0	0	0	. 0	4
01:00 - 01:59	0	4	0	0	0	0	0	0	0	0	0	0	0	4
02:00 - 02:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 - 03:59	0	3	1	0	0	0	0	0	0	0	0	0	. 0	4
04:00 - 04:59	0	11	2	0	0	0	0	0	0	0	0	0	0	13
05:00 - 05:59	0	20	3	0	0	0	0	0	0	0	0	0	0	23
06:00 - 06:59	0	78	14	1	4	0	0	0	0	0	0	0	0	97
07:00 - 07:59	1	72	15	2	7	0	0	0	0	0	0	0	0	97
08:00 - 08:59	0	100	16	1	3	0	1	0	0	0	0	0	0	121
09:00 - 09:59	0	115	18	3	11	1	0	0	0	0	0	0	0	148
10:00 - 10:59	0	85	20	1	5	0	0	0	0	0	0	0	0	111
Totals	7	1790	349	18	158	3	1	1	0	0	0	0	0	2327
Percent of Total	0.3	76.9	15.0	0.8	6.8	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Percent of AM	0.1	79.2	14.3	1.0	5.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
Percent of PM	0.4	75.8	15.4	0.7	7.6	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	100
Total Comment														

Truck Summary:

Total Trucks: 181

% Trucks: 7.8

AM % Trucks: 6.4

PM % Trucks: 8.5

Classification Scheme: FHWA (ID: 1)

#1 Motorcycles - 2 Axles

#2 Passenger Cars - 2 Axles

#3 Pickup Trucks, Vans - 2 Axles

#4 Buses

#5 Single Unit - 2 Axles, 6 Tires

#6 Single Unit Truck - 3 Axles

#7 Single Unit - 4 Axles

#8 Single Unit - 4 Axles or Less

#9 Double Unit - 5 Axles

#10 Double Unit - 6 Axles or More

#11 Multi-Unit - 5 Axles or Less

#12 Multi-Unit - 6 Axles

#13 Multi-Unit - 7 Axles or More

Daily Vehicle Volume Report

Study Date: Wednesday, 03/29/2017 / Thursday, 03/30/2017

Unit ID: 3

Location: Old Frontier Rd. S. of Greaves Way 94.2

	Southbound		Total
	Volume	Volume	Volume
11:00 - 11:59	235	230	465
12:00 - 12:59	270	236	506
13:00 - 13:59	290	215	505
14:00 - 14:59	321	322	643
15:00 - 15:59	519	331	850
16:00 - 16:59	485	381	866
17:00 - 17:59	402	312	714
18:00 - 18:59	297	245	542
19:00 - 19:59	241	128	369
20:00 - 20:59	178	123	301
21:00 - 21:59	89	46	135
22:00 - 22:59	46	28	74
23:00 - 23:59	38	25	63
00:00 - 00:59	21	10	31
01:00 - 01:59	7	8	15
02:00 - 02:59	4	7	11
03:00 - 03:59	. 7	11	18
04:00 - 04:59	13	32	45
05:00 - 05:59	45	151	196
06:00 - 06:59	151	294	445
07:00 - 07:59	261	366	627
08:00 - 08:59	226	284	510
09:00 - 09:59	. 236	223	459
. 10:00 - 10:59	170	188	358
Totals	4552	4196	8748
AM Peak Time	06:44 - 07:43	06:51 - 07:50	06:52 - 07:51
AM Peak Volume	285	380	662
PM Peak Time	15:32 - 16:31	14:41 - 15:40	15:31 - 16:30
PM Peak Volume	557	394	911

Printed: 08/13/2019 at 09:50 TrafficViewer Pro v1.6.4.124

Daily Northbound Classes Report

Study Date: Wednesday, 03/29/2017 / Thursday, 03/30/2017

Unit ID: 3

Location: Old Frontier Rd. S. of Greaves Way 94.2

_								40	#9	#10	#11	#12	#13	Total
	#1	#2	#3	#4	#5	#6	#7	#8	# 9	0	0	0	0	230
11:00 - 11:59	1	183	25	1	19	1	0	0	0	0	0	- 0	0	236
12:00 - 12:59	0	187	29	2	17	1	0	0	0	0	0	0	0	215
13:00 - 13:59	0	147	30	5	31	1	0	0	0	0	0	0	0	322
14:00 - 14:59	1	265	32	3	21	0	0		. 0	0	0	0	. 0	331
15:00 - 15:59	1	271	37	0	20	2	0	0	0	0	0	0	0	381
16:00 - 16:59	2	305	53	0	19	1	1	0	0	0	0	0	. 0	312
17:00 - 17:59	1	257	36	0	18	0	0	0	0	0	0	0	0	245
18:00 - 18:59	1	212	23	0	9	0	0	0	0	0	0	0	0	128
19:00 - 19:59	0	109	15	0	4	0	0	0	0	- 0	0	0	0	123
20:00 - 20:59	1	103	18	0	1	0	0	0	0	0	0	0	0	46
21:00 - 21:59	0	41	2	0	3	0	0	0	0	0	0	0	. 0	28
22:00 - 22:59	0	23	4	0	1	0	0	0	0	0	0	0	0	25
23:00 - 23:59	0	20	5	0	. 0	0	0	0	0	0	0	0	0	10
00:00 - 00:59	0	10	0	. 0	0	0	0	0	0	0	0	0	0	8
01:00 - 01:59	0	6	2	0	0	0	0	0	0	0	0	0	0	7
02:00 - 02:59	0	7	0	0	0	0	0	0	0	0	0	0	0	11
03:00 - 03:59	0	10	1	0	0	0	0	0	0	0	0	0	0	32
04:00 - 04:59	1	25	. 3	. 0	3	0	0	0	0	0	0	0	0	151
05:00 - 05:59	3	115	19	1	13	0	0		0	0	0	0	0	294
06:00 - 06:59	1	197	53	6	36	1	0	0	0	0	0	0	0	366
07:00 - 07:59	1	267	53	1	44	0	0	0	1	0	0	0		284
08:00 - 08:59	1	209	40	3	28	1	0	0	1	1	0	0		223
09:00 - 09:59	3	188	19	0	10	1	0		0	0	0	0	0	188
10:00 - 10:59	0	139	32	0	14	2	1	0	2	1	0	0	0	4196
Totals	18	3296	531	22	311	11	2	2	0.0	0.0	0.0	0.0	0.0	100
Percent of Total	0.4	78.6	12.7	0.5	7.4	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Percent of AM	0.6	75.2	13.7	0.7	9.3	0.3	0.1	0,1	0.0	0.1	0.0	0.0	0.0	100
Percent of PM	0.3	81.1	11.9	0.4	6.0	0.2	.0.0	0.0	0.0	0.0	0.0	0.0	L	L

Truck Summary:

Total Trucks: 351

% Trucks: 8.4

AM % Trucks: 10.5

PM % Trucks: 6.7

Classification Scheme: FHWA (ID: 1)

Motorcycles - 2 Axles

Passenger Cars - 2 Axles

#3 Pickup Trucks, Vans - 2 Axles

#4

Single Unit - 2 Axles, 6 Tires

#6 Single Unit Truck - 3 Axles

Single Unit - 4 Axles Single Unit - 4 Axles or Less

Double Unit - 5 Axles

Double Unit - 6 Axles or More

#11. Multi-Unit'- 5 Axles or Less

#12 Multi-Unit - 6 Axles

#13 Multi-Unit - 7 Axles or More

Printed: 03/30/2018 at 14:06 TrafficViewer Pro v1.6.4.124

Kitsap County Traffic EngineeringDaily Vehicle Volume Report

Study Date: Thursday, 03/29/2018 / Friday, 03/30/2018

Unit ID: 28

Location: Greaves Way E. of Old Frontier Rd. 110.6

	Northbound	Southbound	Total
	Volume	Volume	Volume
11:00 - 11:59	285	293	578
12:00 - 12:59	362	347	709
13:00 - 13:59	353	346	699
14:00 - 14:59	315	307	622
15:00 - 15:59	380	320	700
16:00 - 16:59	409	350	759
17:00 - 17:59	387	308	695
18:00 - 18:59	292	212	504
19:00 - 19:59	202	165	367
20:00 - 20:59	209	107	316
21:00 - 21:59	89	46	135
22:00 - 22:59	53	26	79
23:00 - 23:59	37	13	. 50
00:00 - 00:59	12	4	16
01:00 - 01:59	12	7	19
02:00 - 02:59	6	4	10
03:00 - 03:59	4	13	17
04:00 - 04:59	12	14	26
05:00 - 05:59	44	43	87
06:00 - 06:59	161	177	338
07:00 - 07:59	191	317	508
08:00 - 08:59	213	190	403
09:00 - 09:59	221	257	478
10:00 - 10:59	224	251	475
Totals	4473	4117	8590
AM Peak Time	11:00 - 11:59	07:00 - 07:59	11:00 - 11:59
AM Peak Volume	· 285	317	578
PM Peak Time	16:11 - 17:10	16:03 - 17:02	16:10 - 17:09
PM Peak Volume	428	361	785

Printed: 08/13/2019 at 09:54 TrafficViewer Pro v1.6.4.124

Daily Southbound Classes Report

Study Date: Thursday, 03/29/2018 / Friday, 03/30/2018

Unit ID: 28

Location: Greaves Way E. of Old Frontier Rd. 110.6

1	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
11:00 - 11:59	#1	217	46	0	30	-"0	0	0	0	0	0	0	0	293
12:00 - 12:59	2	244	48	14	38	0	0	. 1	0	0	0	0	0	347
13:00 - 13:59	3	260	47	2	30	2	0	1	0	0	1	0	0	346
14:00 - 14:59	1	235	47	1	17	4	1	1	0	0	0	0	0	307
15:00 - 15:59	0	232	64	0	22	i l	0	1	0	0	0	0	0	320
16:00 - 16:59	0	282	40	0	25		0	1	0	1	o	0	0	350
17:00 - 17:59	4	240	37	0	25	1	0	1	0	0	0	0	0	308
18:00 - 18:59	0	161	33	0	16	2	0	0	0	0	0	0	0	212
19:00 - 19:59	0	137	16	0	12	0	0	0	0	0	0	0	0	165
20:00 - 20:59	0	92	8	0	7	0	0	0	0	0	0	0	0	107
21:00 - 21:59	0	37	6	0	3	0	0	0	0	0	0	0	0	46
22:00 - 22:59	0	21	3	0	2	0	0	0	0	0	0	0	0	26
23:00 - 23:59	0	12	1	0	0	0	0	0	0	0	0	0	0	13
00:00 - 00:59	0	3	1	0	0	0	0	0	0	0	0	0	0	4
01:00 - 01:59	0	7	0	0	0	0	0	0	0	0	0	0	0	7
02:00 - 02:59	0	4	0	0	0	0	0	0	0	0	0	0	0	4
03:00 - 03:59	0	12	1	0	0	0	0	0	0	0	0	0	0	13
04:00 - 04:59	1	10	1	0	2	0	0	0	0	0	0	0	0	14
05:00 - 05:59	0	27	8	0	8	0	0	0	0	0	0	0	0	43
06:00 - 06:59	0	110	23	10	33	1	0	0	0	0	0	0	. 0	177
07:00 - 07:59	3	226	37	1	43	3	1	2	1	0	0	0	0	317
08:00 - 08:59	2	140	17	1	28	1	0	0	1	0	0	0	0	190
09:00 - 09:59	1	180	48	0	21	4	1	2	0	0	0	0	0	257
10:00 - 10:59	0	178	41	1	24	0	0	2	2	1	2	0	0	251
Totals	17	3067	573	30	386	20	3	12	4	2	3	0	0	4117
Percent of Total	0.4	74.5	13.9	0.7	9.4	0.5	0.1	0.3	0.1	0.0	0.1	0.0	0.0	100
Percent of AM	0.4	71.0	14.2	0,8	12.0	0.6	0.1	0.4	0.3	0.1	0.1	0.0	0.0	100
Percent of PM	0.4	76.7	13.7	0.7	7.7	0.4	0.0	0.2	0.0	0.0	0.0	0.0	0.0	100
T LO	0.1						1	1		······································				

Truck Summary:

Total Trucks: 460

% Trucks: 11.2

AM % Trucks: 14.4

PM % Trucks: 9.2

Classification Scheme: FHWA (ID: 1)

#1 Motorcycles - 2 Axles

#2 Passenger Cars - 2 Axles

#3 Pickup Trucks, Vans - 2 Axles

#4 Buses

#5 Single Unit - 2 Axles, 6 Tires

#6 Single Unit Truck - 3 Axles

#7 Single Unit - 4 Axles

#8 Single Unit - 4 Axles or Less

#9 Double Unit - 5 Axles

#10 Double Unit - 6 Axles or More

#11 Multi-Unit - 5 Axles or Less

#12 Multi-Unit - 6 Axles

#13 Multi-Unit - 7 Axles or More

Appendix B. Peak Hour Turning Movement Count Data

This is your address Your City, State, Zip Code Your Tagline Here

Old Frontier Rd. & Greaves Way Turning Movement Count 4:00 to 6:00 PM October

File Name: OLD FRONTIER-OF GREAVES_11-15-17_TM_0 Site Code: 000000000

Start Date : 10/26/2017 Page No : 1

	Int. Total	266	278	289	270	1103	308	288	263	243	1102	2205			2205	100	0	
	App. Total	93	88	101	77	360	107	81	94	92	374	734		33.3	734	100	0	c
OLD FRONTIER RD. From West	Thru	71	64	70	25	257	73	61	65	63	262	519	70.7	23.5	519	100	0	c
OLD	Left	22	72	31	25	103	34	20	29	29	112	215	29.3	9.8	215	100	0	c
	App. Total	95	116	116	133	460	132	135	112	26	476	936		42.4	936	100	0	_
WAY ast	Right	24	27	20	46	117	30	23	22	24	66	216	23.1	9.8	216	100	0	c
GREAVES WAY From East	Thru	71	88	96	87	342	102	112	90	73	377	719	76.8	32.6	719	100	0	c
	Peds	0	_	0	0	-	0	0	0	0	0	_	0.1	0	-	100	0	C
).	App. Total	78	73	72	09	283	69	72	22	54	252	535		24.3	535	100	0	-
OLD FRONTIER RD From North	Right	56	45	51	38	190	39	44	38	32	153	343	64.1	15.6	343	100	0	c
OLD F	Left	22	28	21	22	93	30	28	19	22	66	192	35.9	8.7	192	100	0	c
	Start Time	04:00 PM	04:15 PM	04:30 PM	04:45 PM	Total	05:00 PM	05:15 PM	05:30 PM	05:45 PM	Total	Grand Total	Apprch %	Total %	Unshifted	% Unshifted	Bank 1	0/ Donly 4

This is your address Your City, State, Zip Code Your Tagline Here

Old Frontier Rd. & Greaves Way

Turning Movement Count

4:00 to 6:00 PM

October

: OLD FRONTIER-OF GREAVES_11-15-17_TM_0 File Name

Site Code

: 00000000 : 10/26/2017 : 2 Start Date Page No

Total 1647 0 1647 216 0 216 Right 719 0 719 Thru Total 10/26/2017 04:00 PM 10/26/2017 05:45 PM 343 192 0 0 343 192 Right Left North Unshifted Bank 1 71€ 71€ 71€ 71€ 918 0 918 04T 1062 1062 1062 1062

This is your address Your City, State, Zip Code Your Tagline Here

Old Frontier Rd. & Greaves Way

Turning Movement Count

October

4:00 to 6:00 PM

File Name: OLD FRONTIER-OF GREAVES_11-15-17_TM_0 Site Code: 000000000

Start Date : 10/26/2017 Page No : 3

	Int. Total			289	270	308	288	1155		.938
	App. Total			101	177	107	84	366	5 45000	.855
OLD FRONTIER RD. From West	Thru			70	52	73	61	256	6.69	.877
OLD F	Left			31	25	34	20	110	30.1	.809
	App. Total			116	133	132	135	516		.956
WAY	Right			20	46	30	23	119	23.1	.647
GREAVES WAY From East	Thru			96	87	102	112	397	76.9	.886
	Peds			0	0	0	0	0	0	000.
-	App. Total			72	09	69	72	273		.948
OLD FRONTIER RD. From North	Right	- Peak 1 of 1	PM (51	38	38	44	172	63	.843
OLD F	Left	PM to 05:45 PM	Begins at 04:30	21	22	30	28	101	37	.842
	Start Time	Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of	Peak Hour for Entire Intersection Begins at 04:30 PM	04:30 PM	04:45 PM	05:00 PM	05:15 PM	Total Volume	% App. Total	PHF

This is your address
Your City, State, Zip Code
Your Tagline Here

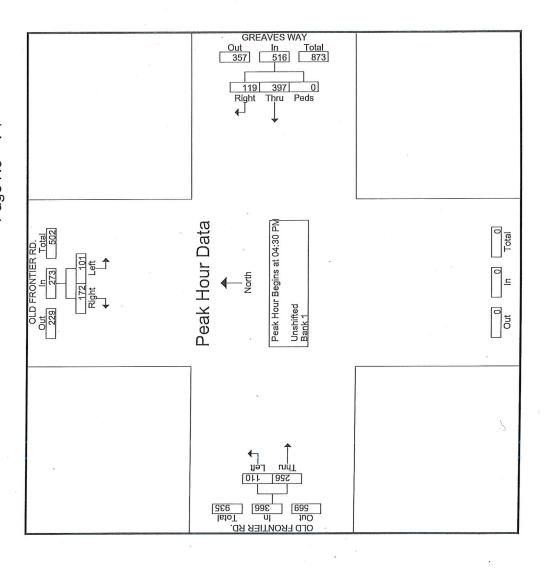
Old Frontier Rd. & Greaves Way

Turning Movement Count

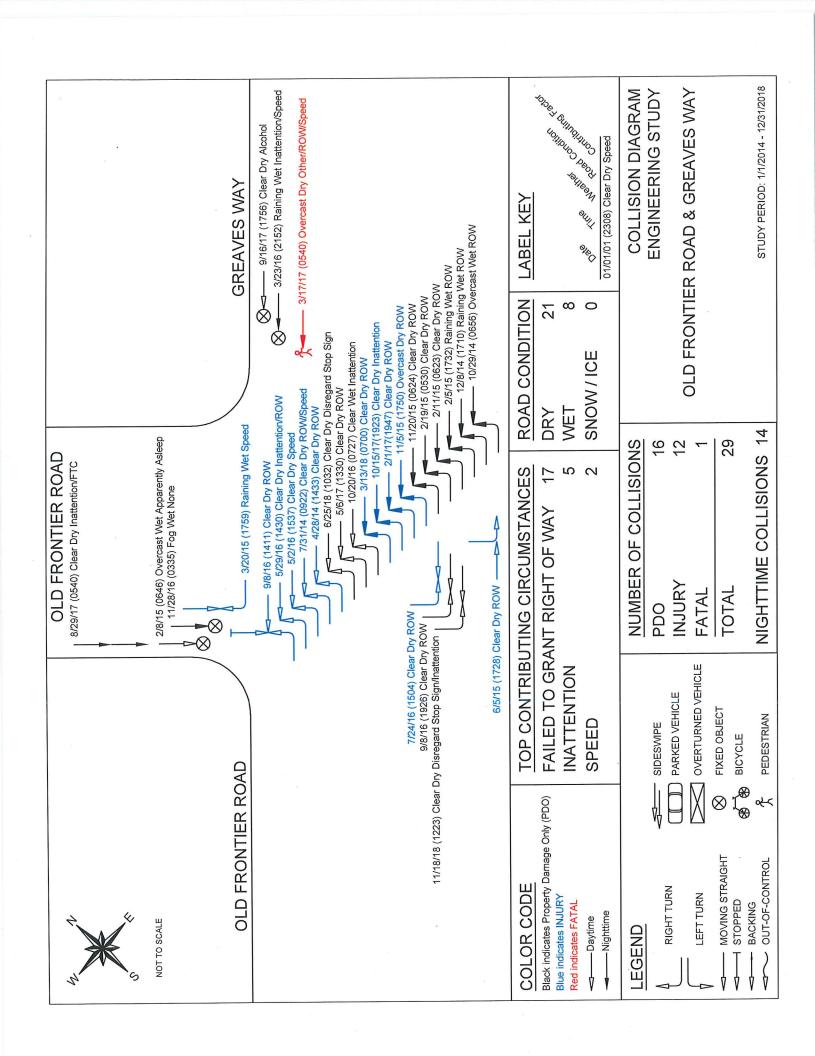
4:00 to 6:00 PM

October

File Name: OLD FRONTIER-OF GREAVES_11-15-17_TM_0 Site Code: 000000000 Start Date: 10/26/2017 Page No: 4



Appendix C. Collision Diagram



Appendix D. Signal Warrant Worksheet



Kitsap County Department of Public Works - Traffic Operations 614 Division Street, MS-26, Port Orchard, WA 98366

TRAFFIC SIGNAL VOLUME WARRANT ANALYSIS

Intersection name:

Greave Way & Old Frontier

Date: 4/5/2018

Major street name:

Number of approache lanes:

Posted speed (mph):

Minor street name:

Number of approache lanes: Posted speed (mph):

Count year:

Analysis year:

Annual growth rate (%):

Growth factor:

Greave Way 2 35

1.5

1.00

Volume Criteria

Warrant	Major	Minor
1A-Met	420	140
1B	630	70
1A & 1B	Major	Minor
1A	336	112
1B	504	56
2	890	80
3B	1183	100

Peak hour:

4:00 PM

Isolated community with population < 10,000 ? 85th percentile speed > 40 mph on major street? No Yes

The state of the s	Minor R Highe Hours Major Total Approa		Met Warrant 1A	Not Met Warrant 1B	- I was to the same of the sam	let ion 1A & 1B	Met Warrant 2	Met Warrant 3E
Hours			Met	Met	Met	Met	Met	Met
12:00 AM	14	Approach 4	No	No.	No	. No	No	No
12:00 AM	15	4	No	No	No	No	No	i\lo
12:30 AM	13	5	No	No	No	No	No	No
12:45 AM	13	4	No	No	No	No	No	. No
1:00 AM	15	4	iNo	No	No	No	No	No
1:15 AM	13	4	No	No	No	No	No	No
1:30 AM	14	2	No	No	No	No	No	No
1:45 AM	12	1	No	No	No	No	No	No
2:00 AM	11	. 0	No	No	No	No	· No	- No
2:15 AM	11	-1	No	No	No	No ·	No · ·	No
2:30 AM	10	2	No	No	No	No	No	No
2:45 AM	16	3	No	No	No	No	No	No
3:00 AM	24	4	No	No	No	No	No	No
3:15 AM	28	5	No	No	No	No	No	No
3:30 AM	33	7	No	No	No	No	No	оVi
3:45 AM	36 .	10	No	No	No	No	No	No
4:00 AM	46	13	No	No	No	No	No	No
4:15 AM	72	13	No	No	No	· No	No	No
4:30 AM	111	14	No	No	No	No	No	No
4:45 AM	145	18	No	No	No	No	No	No
5:00 AM	194	23	No	No -	No ·	No	No	No -
5:15 AM	256	35	No	No	No	No	No	No
5:30 AM	320	47	No	No	No	No	No	No
5:45 AM	400	56	No	No	No	No	No	No
6:00 AM	471	97	No	No	No	No ·	No	No
6:15 AM	535	101	No	No	No	Yes	No	No
6:30 AM	610	109	No	No	No		No	No
6:45 AM	662	122	No	Yes	Yes		No	No
7:00 AM	683 .	97	No				No	No
7:15 AM	625	104	No			Yes	No	No
7:30 AM	548	113	No				No	No
7:45 AM	502	111	No	No	No		No	No
8:00 AM	474	121	No	No	Yes		No	No
8:15 AM	504	150	Yes	No		Yes	No	No
8:30 AM	505	146		No			No	No
8:45 AM		143		No			No	No
9:00 AM		148		No	Yes		Mo	No
9:15 AM		113	No	No		No	No	No
9:30 AM		119	No	No		No	No	No
9:45 AM		120	No	No		No	- No	No

10:00 AM	439	111	No .	No	No	No	No	No
10:15 AM	447	139	No	No	Yes	No	No	No
10:30 AM	496	148	Yes	No		No	No	No
10:45 AM	519	155		. No		Yes	No	No
11:00 AM	523	175		No			No	1/10
11:15 AM	538	162		No	Yes .		No	No
11:30 AM	557	161	Yes	No			No	No
11:45 AM	576	172		No		Yes	No	No
12:00 PM	583	150		No			No	No
12:15 PM	587	154		No	Yes		.No	No
12:30 PM	571	142	Yes	No			No	No
12:45 PM	562	135		No		Yes	No	. No
1:00 PM[561	147		No			No	No
1:15 PM	567	143		No	Yes		No	No
1:30 PM	566	154	Yes	No	•		No	No
1:45 PM	583	162		:No		Yes	No	No
2:00 PM	629	180		No			Yes	No
2:15 PM	666	198		Yes	Yes			No
2:30 PM	697	223	Yes					No
2:45 PM	688	264				Yes		Yes
3:00 PM	651	285					Yes	
3:15 PM	652	294		Yes	Yes			
3:30 PM	672	303	Yes					
3:45 PM	692	284				Yes		Yes
4:00 PM	731	257					Yes	
4:15 PM	725	250		Yes	Yes			
4:30 PM	700	221	Yes					
4:45 PM	675	201				Yes		No
5:00 PM	620	186					Yes	No
5:15 PM	557	168		No	Yes			No
5:30 PM	520	160	Yes	No				No
5:45 PM	494	149		No		No		No
6:00 PM	457	147		No		No	No	No
6:15 PM	426	135		No	Yes	No	No	No-
6:30 PM	391	137	No	No		No	No	No
6:45 PM	321	120	No	No		No	No	No
7:00 PM	293	98	····No	No	-	No.	No	. No .
7:15 PM	274	82	No	No	No	No	No .	No
7:30 PM	237	54	No	No	No	No	No	No
7:45 PM	243	50	No	No	No	No	No	No
8:00 PM	230	47	No	No	No	No	. No	No
8:15 PM	187	44	No	No	No	No	No	No
8:30 PM	153	34	No	No	No	No	No	No
8:45 PM	124	20	No	No	No	No	No	No No
9:00 PM	92	17	No	No	No	No	No No	No
9:15 PM	77	12	No	No	No	No		No
9:30 PM	76	15	No	No	No	No No	No . No	No
9:45 PM	67	16	No	No	No	No	. No	No
10:00 PM	54	10	No	.No	. No	No -	No	No
10:15 PM	56	12	No	No	No No	No No	No	No
10:30 PM	53	9	No	No No	No No	No No	No	No
10:45 PM	39	7	No		No	No	No	No
11:00 PM	38	6	No	No No	No	No	No	No
11:15 PM	28	4	No		No	No	No	No
11:30 PM	19 17	2 4	No .	No No	No	No	No	No
11:45 PM	17							2
	0.11	Total	9 8 hours	. 4	12	10	4	
	Criteria			8 hours		r 1A and 1B	4 Hours	1 Hour
Wa	Warrant Summary			Warrant 1B	Warrant 1A		Warrant 2	Warrant 3B Met
	Warrant Summary			Not Met		let	Met	

Warrant 1 - Eight Hour Volume

Condition A - Minimum Vehicular Volume,

Condition B - Interruption of Continous Traffic,

Combination - Warrant 1A and 1B

Warrant 2 - Four Hour Volume - MUTCD, Figure 4C-1 and 4C-2 Warrant 3B - Peak Hour Volume - MUTCD, Figure 4C-3 and 4C-4

Appendix E. AWSC Analysis Worksheets

								•
Intersection					STEW.			
Intersection Delay, s/veh	18				240			
Intersection LOS	C							
Mayamant	SBL	SBR	NEL	NET	SWT	SWR		
Movement	SBL	SBR	NEL		5W1	3001		Lange Control
Lane Configurations					T 397	119		
Traffic Vol, veh/h	101	172	110	256		119		
Future Vol, veh/h	101	172	110	256	397			
Peak Hour Factor	0.84	0.84	0.81	0.88	0.89	0.65		
Heavy Vehicles, %	2	2	2	2	2	402		
Mvmt Flow	120	205	136	291	446	183		
Number of Lanes	1	1	1	- 1	1	1		
Approach	SB		NE		SW			
Opposing Approach			SW		NE			
Opposing Lanes	0		2		2			
Conflicting Approach Left	SW		SB					
Conflicting Lanes Left	2		2		0			
Conflicting Approach Right	ΝE				SB			
Conflicting Lanes Right	2		0		2			
HCM Control Delay	13		15		22.6			
HCM LOS	В		В		C			
		NEL nd	NEL 50	SBLn1	SBLn2	SWLn1	SWLn2	
Lane		NELn1	NELn2			0%	0%	
Vol Left, %		100%	0%	100%	0%		0%	
Vol Thru, %		0%	100%	0%	0%	100%		
Vol Right, %		0%	0%	0%	100%	0%	100%	
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane		110	256	101	172	397	119	
LT Vol		110	0	101	0	0	0	
Through Vol		0	256	0	0	397	0	
RT Vol		0	0	0	172	0	119	
Lane Flow Rate		136	291	120	205	446	183	
Geometry Grp		7	7	7	7	7	7	
Degree of Util (X)		0.264	0.524	0.256	0.366	0.776	0.282	
Departure Headway (Hd)		6.993	6.484	7.66	6.437	6.263	5.551	
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes	
Cap		512	553	468	557	575	644	
Service Time		4.762	4.252	5.428	4.204	4.025	3.312	
HCM Lane V/C Ratio		0.266	0.526	0.256	0.368	0.776	0.284	
HCM Control Delay		12.3	16.2	13.1	12.9	27.5	10.5	
HCM Lane LOS		В	C	В	В	D	В	
HCM 95th-tile Q		1.1	3	1	1.7	7.2	1.2	

Movement	SB	SB	NE	NE	SW	
Directions Served	L	R	L	T	T	
Maximum Queue (ft)	64	70	75	100	213	
Average Queue (ft)	29	32	37	55	91	
95th Queue (ft)	49	57	62	84	166	
Link Distance (ft)		633		423	917	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)	100		100			
Storage Blk Time (%)	0	0	0	0		
Queuing Penalty (veh)	0	0	0	0		

Network Summary

Intersection Delay, s/veh 124.5 Intersection LOS
Intersection Delay, s/veh Intersection LOS
Movement SBL SBR NEL NET SWT SWR Lane Configurations Taraffic Vol, veh/h 169 287 184 428 663 199 Future Vol, veh/h 169 287 184 428 663 199 Future Vol, veh/h 169 287 184 428 663 199 Future Vol, veh/h 169 287 184 428 663 199 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Feak Hour Factor 0.84 0.84 0.84 0.84 0.89 0.89 0.65 Feak Hour Factor 0.84 0.84 0.84 0.89 0.89 0.65 Feak Hour Factor 0.84 0.84 0.84 0.89 0.89 0.65 Feak Hour Factor 0.84 0.84 0.89 0.89 0.65 Feak Hour Factor 0.84 0.84 0.84 0.84 0.84 0.89 0.89 0.65 Feak Hour Factor 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.89 Feak Hour Factor 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.8
Lane Configurations
Lane Configurations
Lane Configurations
Traffic Vol, veh/h 169 287 184 428 663 199 Future Vol, veh/h 169 287 184 428 663 199 Peak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 306 Number of Lanes 8 NE SW SB Conflicting Lanes Left 2 2 0 2 2 H
Future Vol, veh/h 169 287 184 428 663 199 Peak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 306 Number of Lanes 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Peak Hour Factor 0.84 0.84 0.81 0.88 0.89 0.65 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 306 Number of Lanes 0 2 22 486 745 306 Number of Lanes 0 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <td< td=""></td<>
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2
Mymt Flow 201 342 227 486 745 306 Number of Lanes 1 1 1 1 1 1 1 1 Approach SB NE SW NE Opposing Approach SW NE Opposing Lanes 0 2 2 2 2 Conflicting Approach Left SW SB SB Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB SB SB Conflicting Lanes Right 2 0 2 2 2 0 2 2 2 10 2 2 2 10 2 2 10 2 2 10 2 2 2 10 2 2 10 2 2 10 2 2 10 2 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10
Number of Lanes 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Approach SB NE SW Opposing Approach SW NE Opposing Lanes 0 2 2 Conflicting Approach Left SW SB Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 0% 0% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop
Opposing Approach SW NE Opposing Lanes 0 2 2 Conflicting Approach Left SW SB Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F **Page 14.** **Lane** **NELn1** NELn2** SBLn1** SBLn2** SWLn1** SWLn2** **Vol Left, %** **Vol Left, %** **100%** 0%** 100%** 0%** 0%** 0%** 0%**
Opposing Lanes 0 2 2 Conflicting Approach Left SW SB Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Conflicting Approach Left SW SB Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2
Conflicting Lanes Left 2 2 0 Conflicting Approach Right NE SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2
Conflicting Approach Right NE SB Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Conflicting Lanes Right 2 0 2 HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
HCM Control Delay 25.1 61.9 218.4 HCM LOS D F F Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop
Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Lane NELn1 NELn2 SBLn1 SBLn2 SWLn1 SWLn2 Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 100% 0% 100% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Vol Left, % 100% 0% 100% 0% 0% Vol Thru, % 0% 100% 0% 0% 100% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Vol Left, % 100% 0% 100% 0% 0% 0% Vol Thru, % 0% 100% 0% 0% 100% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Vol Thru, % 0% 100% 0% 100% 0% Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop
Vol Right, % 0% 0% 0% 100% 0% 100% Sign Control Stop Stop Stop Stop Stop Stop Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
Sign Control Stop
Traffic Vol by Lane 184 428 169 287 663 199 LT Vol 184 0 169 0 0 0
LT Vol 184 0 169 0 0 0
Through voi
RT Vol 0 0 0 287 0 199
Lane Flow Rate 227 486 201 342 745 306
Geometry Grp 7 7 7 7 7 7
Degree of Util (X) 0.514 1.033 0.481 0.703 1.6 0.596
Departure Headway (Hd) 8.861 8.343 9.347 8.103 7.733 7.011
Convergence, Y/N Yes Yes Yes Yes Yes Yes
Cap 409 438 388 451 469 511
Service Time 6.561 6.043 7.047 5.803 5.53 4.807
HCM Lane V/C Ratio 0.555 1.11 0.518 0.758 1.588 0.599
HCM Control Delay 20.6 81.2 20.4 27.8 300 19.7
HCM Control Delay 20.6 81.2 20.4 27.8 300 19.7 HCM Lane LOS C F C D F C

Movement	SB	SB	NE	NE	SW	SW
Directions Served	- L	R	L	T	T	R
Maximum Queue (ft)	124	209	125	302	964	956
Average Queue (ft)	70	87	73	118	936	933
95th Queue (ft)	124	169	125	222	952	946
Link Distance (ft)		633		423	917	917
Upstream Blk Time (%)				0	99	80
Queuing Penalty (veh)				0	0	0
Storage Bay Dist (ft)	100		100			
Storage Blk Time (%)	2	9	2	11		
Queuing Penalty (veh)	6	15	10	20		

Network Summary

Appendix F. Signalized Analysis Worksheets

	4	لر	Ť	×	K	t			
Movement	SBL	SBR	NEL	NET	SWT	SWR			
Lane Configurations	ħ	71	7	^	^	71			
Traffic Volume (veh/h)	101	172	110	256	397	119			
Future Volume (veh/h)	101	172	110	256	397	119			
Initial Q (Qb), veh	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No	1100		No	No				
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870			
Adj Flow Rate, veh/h	120	205	136	291	446	0			
Peak Hour Factor	0.84	0.84	0.81	0.88	0.89	0.65			
Percent Heavy Veh, %	2	2	2	2	2	2			
Cap, veh/h	346	308	217	1054	600				
Arrive On Green	0.19	0.19	0.12	0.56	0.32	0.00			
Sat Flow, veh/h	1781	1585	1781	1870	1870	1585			
Grp Volume(v), veh/h	120	205	136	291	446	0			
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1870	1870	1585			
Q Serve(g_s), s	2.2	4.4	2.7	3.0	7.9	0.0			
Cycle Q Clear(g_c), s	2.2	4.4	2.7	3.0	7.9	0.0			
Prop In Lane	1.00	1.00	1.00	0.0	7.0	1.00			
ane Grp Cap(c), veh/h	346	308	217	1054	600	1.00			
//C Ratio(X)	0.35	0.67	0.63	0.28	0.74				
Avail Cap(c_a), veh/h	839	747	408	1687	1032				
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Jpstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00			
Jniform Delay (d), s/veh	12.9	13.9	15.5	4.2	11.3	0.0			
ncr Delay (d2), s/veh	0.6	2.5	3.0	0.1	1.9	0.0			
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%),veh/ln	0.7	0.2	1.1	0.5	2.6	0.0			
Jnsig. Movement Delay, s/vel		0.2	101	3.0	2,0	3,0			
_nGrp Delay(d),s/veh	13.5	16.3	18.5	4.3	13.1	0.0			
_nGrp LOS	В	В	В	A.0	В	0,0			
Approach Vol, veh/h	325			427	446	А	4 1 1 1 1 1		7.5
Approach Vol, ven/n Approach Delay, s/veh	15.3			8.8	13.1	11			
Approach LOS	15.5 B			Α	13.1 B				
	D					_	-	0	
Timer - Assigned Phs				4		6	7	8	
Phs Duration (G+Y+Rc), s				25.4		11.7	9.0	16.4	
Change Period (Y+Rc), s				4.5		4.5	4.5	4.5	
Max Green Setting (Gmax), s				33.5		17.5	8.5	20.5	
Max Q Clear Time (g_c+l1), s				5.0		6.4	4.7	9.9	
Green Ext Time (p_c), s				1.7		0.8	0.1	2.0	4 10
Intersection Summary									
HCM 6th Ctrl Delay			12.2						
HCM 6th LOS			В						
Notes		3,457							

Unsignalized Delay for [SWR] is excluded from calculations of the approach delay and intersection delay.

Movement	SB	SB	NE	NE	SW
Directions Served	L	R	L	Т	T
Maximum Queue (ft)	86	94	108	133	200
Average Queue (ft)	36	38	49	48	101
95th Queue (ft)	71	73	85	99	161
Link Distance (ft)		633		423	917
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	100		100		
Storage Blk Time (%)	0	0	1	0	
Queuing Penalty (veh)	0	0	2	0	

Network Summary

	W	لير	Ť	×	K	₺			
Movement	SBL	SBR	NEL	NET	SWT	SWR			
Lane Configurations	7	71	7	^	1	7			
Traffic Volume (veh/h)	169	287	184	428	663	199			
Future Volume (veh/h)	169	287	184	428	663	199			
nitial Q (Qb), veh	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No	1100		No	No				
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870			
Adj Flow Rate, veh/h	201	342	227	486	745	0			
Peak Hour Factor	0.84	0.84	0.81	0.88	0.89	0.65			
Percent Heavy Veh, %	2	2	2	2	2	2			
Cap, veh/h	425	378	268	1201	808				
Arrive On Green	0.24	0.24	0.15	0.64	0.43	0.00			
Sat Flow, veh/h	1781	1585	1781	1870	1870	1585			
Grp Volume(v), veh/h	201	342	227	486	745	0			
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1870	1870	1585			
Q Serve(g_s), s	7.3	15.8	9.4	9.5	28.3	0.0			
Cycle Q Clear(g_c), s	7.3	15.8	9.4	9.5	28.3	0.0			
Prop In Lane	1.00	1.00	1.00	0.0	20.0	1.00			
ane Grp Cap(c), veh/h	425	378	268	1201	808	1.00			
	0.47	0.91	0.85	0.40	0.92				
//C Ratio(X)	437	389	295	1303	881				
Avail Cap(c_a), veh/h HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Jpstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00			
	24.6	27.9	31.2	6.5	20.2	0.0			
Jniform Delay (d), s/veh	0.8	23.7	18.7	0.2	14.2	0.0			
ncr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0			
nitial Q Delay(d3),s/veh	2.9	15.1	5.2	2.9	14.0	0.0			
%ile BackOfQ(50%),veh/ln		10.1	5.2	2.5	14.0	0.0			
Insig. Movement Delay, s/veh		51 G	49.9	6.7	34.4	0.0			
nGrp Delay(d),s/veh	25.5	51.6				0.0			
nGrp LOS	CC	D	D	A 742	74E	٨			
Approach Vol, veh/h	543			713	745	Α			
Approach Delay, s/veh	41.9	April 1980		20.5	34.4				
Approach LOS	D			C	C				
Γimer - Assigned Phs				4		6	7	8	
Phs Duration (G+Y+Rc), s				52.9		22.5	15.8	37.1	
Change Period (Y+Rc), s				4.5		4.5	4.5	4.5	
Max Green Setting (Gmax), s		92.4		52.5		18.5	12.5	35.5	
Max Q Clear Time (g_c+l1), s				11.5		17.8	11.4	30.3	
Green Ext Time (p_c), s				3.3		0.2	0.1	2.2	
Intersection Summary	A7638				162000				
HCM 6th Ctrl Delay		NICHT:	31.5	19.7-2					
HCM 6th LOS			C C						
			٥						
otes									5-10-1

Unsignalized Delay for [SWR] is excluded from calculations of the approach delay and intersection delay.

Movement	SB	SB	NE	NE	SW
Directions Served	L	R	L	Т	Т
Maximum Queue (ft)	124	276	124	343	449
Average Queue (ft)	79	108	94	110	235
95th Queue (ft)	135	205	135	248	399
Link Distance (ft)		633		423	917
Upstream Blk Time (%)				0	
Queuing Penalty (veh)				0	
Storage Bay Dist (ft)	100		100		
Storage Blk Time (%)	4	7	13	3	
Queuing Penalty (veh)	11	11	54	6	

Network Summary

Appendix G. Roundabout Analysis Worksheets

LANE LEVEL OF SERVICE

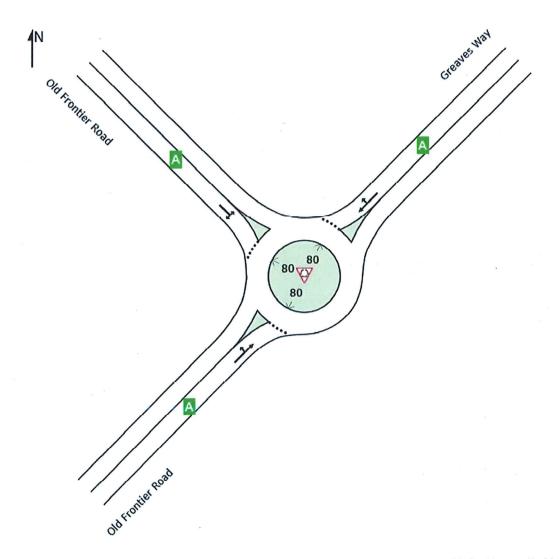
Lane Level of Service

Lane Level of Service

♥ Site: 101 [2017_Old Frontier Rd & Greaves Way-all single approach]

Old Frontier Rd & Greaves Way Site Category: (None) Roundabout

		Approaches		Intersection
	Northeast	Northwest	Southwest	Intersection
LOS	Α	Α	Α	Α



Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

∜ Site: 101 [2017_Old Frontier Rd & Greaves Way-all single approach]

Old Frontier Rd & Greaves Way Site Category: (None)

Roundabout

	Service of the service of	erformanc					050/ 5	40		Ett. C	A Ma	A
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance ft	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
North	East: Gre	aves Way										
6x	T1	397	3.0	0.453	4.0	LOS A	3.0	77.9	0.38	0.45	0.38	33.
16x	R2	119	3.0	0.453	4.2	LOS A	3.0	77.9	0.38	0.45	0.38	33.
Appro	ach	516	3.0	0.453	4.1	LOS A	3.0	77.9	0.38	0.45	0.38	33.
North	West: Old	Frontier Ro	ad									
7x	L2	101	3.0	0.302	11.9	LOS B	1.7	43.5	0.58	0.72	0.58	33.
14x	R2	172	3.0	0.302	6.5	LOS A	1.7	43.5	0.58	0.72	0.58	32.
Approach		273	3.0	0.302	8.5	LOS A	1.7	43.5	0.58	0.72	0.58	32
South	West: Old	d Frontier Ro	oad									
5x	L2	110	3.0	0.321	9.0	LOS A	2.0	50.2	0.33	0.49	0.33	33.
2x	T1	256	3.0	0.321	3.8	LOSA	2.0	50.2	0.33	0.49	0.33	32.
Appro	ach	366	3.0	0.321	5.4	LOSA	2.0	50.2	0.33	0.49	0.33	32
All Ve	hicles	1155	3.0	0.453	5.5	LOSA	3.0	77.9	0.41	0.53	0.41	33.

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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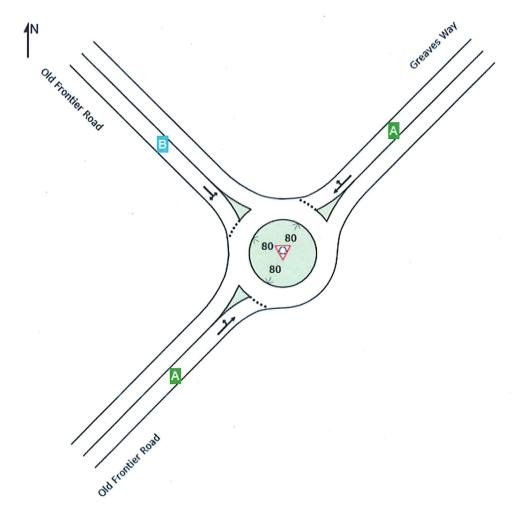
LANE LEVEL OF SERVICE

Lane Level of Service

\overline{\text{V}} Site: 101 [2043_Old Frontier Rd & Greaves Way-all single approach]

Old Frontier Rd & Greaves Way Site Category: (None) Roundabout

		Intersection		
	Northeast	Northwest	Southwest	Intersection
LOS	Α	В	Α	Α



Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

❤️ Site: 101 [2043_Old Frontier Rd & Greaves Way-all single approach]

Old Frontier Rd & Greaves Way Site Category: (None) Roundabout

Mov	Tiurn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID) Gilli	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate		Speed
		veh/h	%	v/c	sec		veh	ft				mpl
Northl	East: Grea	aves Way										
6x	T1	663	3.0	0.727	5.4	LOSA	8.1	207.4	0.70	0.61	0.73	32.1
16x	R2	199	3.0	0.727	5.6	LOSA	8.1	207.4	0.70	0.61	0.73	32.7
Appro	ach	862	3.0	0.727	5.5	LOS A	8.1	207.4	0.70	0.61	0.73	32.2
North\	Nest: Old	Frontier Ro	ad									
7x	L2	169	3.0	0.598	16.4	LOS B	5.7	146.6	0.91	1.01	1.12	31.4
14x	R2	287	3.0	0.598	11.1	LOS B	5.7	146.6	0.91	1.01	1.12	30.5
Approach		456	3.0	0.598	13.1	LOS B	5.7	146.6	0.91	1.01	1.12	30.9
South'	West: Old	Frontier Ro	oad									
5x	L2	184	3.0	0.519	9.5	LOS A	4.3	109.2	0.54	0.54	0.54	33.4
2x	T1	428	3.0	0.519	4.4	LOS A	4.3	109.2	0.54	0.54	0.54	32.0
Appro	ach	612	3.0	0.519	5.9	LOSA	4.3	109.2	0.54	0.54	0.54	32.4
All Vel	nicles	1930	3.0	0.727	7.4	LOSA	8.1	207.4	0.70	0.68	0.76	32.0

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Appendix H. Roundabout Conceptual Design

Silverdale, Washington

Parcels

12

100

200